

Ian Smith Consulting

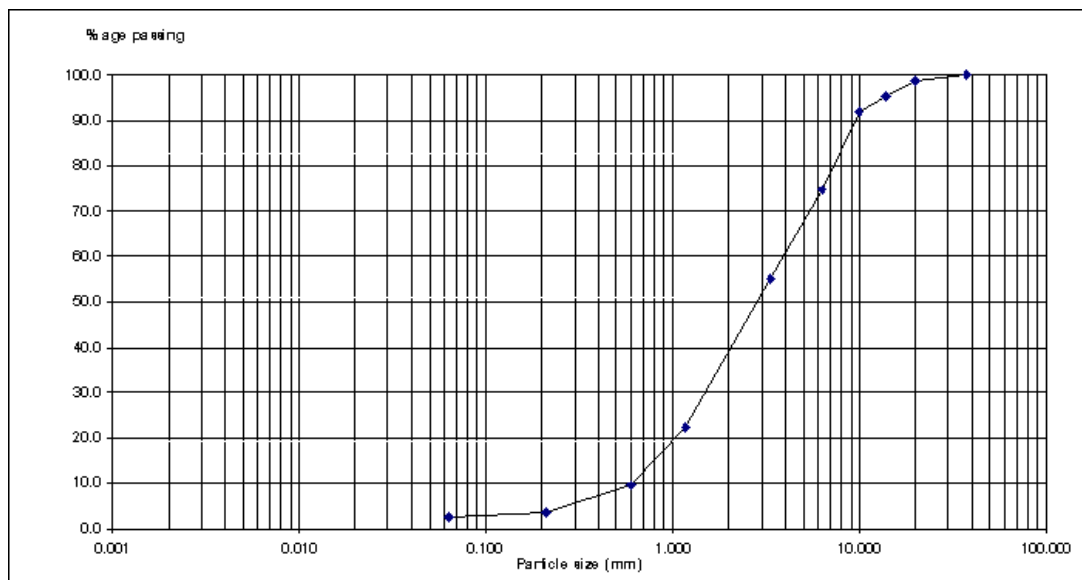
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Solutions to Sample Exam questions – by Professor Ian Smith
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The sample exam questions are provided in a separate downloadable file.

Q1. (a)

Sieve (mm)	Mass (g)	% retained	% passing
37.5	0	0.0	100.0
20.0	2	1.2	98.8
14.0	5.4	3.3	95.5
10.0	6.2	3.7	91.8
6.3	28.3	17.1	74.7
3.35	32.5	19.6	55.0
1.18	54	32.6	22.4
0.60	21	12.7	9.7
0.212	10.2	6.2	3.5
0.063	1.6	1.0	2.5
pan	4.2	2.5	



(10 marks)

(b)

From PSD:

$$D_{60} = 4$$
$$D_{10} = 0.64$$

$$C_u = \frac{D_{60}}{D_{10}} = \frac{4}{0.64} = \underline{\underline{6.25}}$$

(2 marks)

(c)

$$(i) \quad \rho_b = \frac{M}{V} = \frac{2.035 \times 10^{-3}}{1.05 \times 10^{-3}} = \underline{\underline{1.938 \text{ Mg/m}^3}}$$

$$(ii) \quad w = \frac{\text{mass of water}}{\text{mass of dry soil}} = \frac{2.035 - 185}{185} = \underline{\underline{10\%}}$$

$$(iii) \quad \rho_d = \frac{\rho_b \times 100}{100 + w} = \frac{1938}{110} = \underline{\underline{1.762 \text{ Mg/m}^3}}$$

$$(iv) \quad \rho_d = \frac{\rho_w G_s}{1 + e} \quad (\text{N.B. } \rho_w = \text{density of water} = 1 \text{ Mg/m}^3)$$

$$\Rightarrow 1.762 = \frac{2.70}{1 + e}$$

$$e = 1.53 - 1 = \underline{\underline{0.53}}$$

$$(v) \quad \rho_b = \frac{G_s + eS_r}{1 + e}$$

$$1.938 = \frac{2.70 + 0.53S_r}{1.53}$$

$$0.53S_r = (1.938 \times 1.53) - 2.7$$

$$S_r = \frac{0.265}{0.53} = 0.50 \equiv 50\%$$

(13 marks)

(Total 25 marks)

Q2.

(a)

(i) Consider two 30 m x 4 m rectangles meeting over point A:

$$m = b / z = 4 / 3 = 1.33$$

$$n = l / z = 30 / 3 = 10$$

From chart, $I_o = 0.222$

$$\Rightarrow \text{applied stress} = 2 \times 0.222 \times 40 = \underline{17.8 \text{ kPa}}$$

(ii) Consider four 15 m x 4 m rectangles meeting over point A:

$$m = b / z = 4 / 3 = 1.33$$

$$n = l / z = 15 / 3 = 5$$

From chart, $I_o = 0.222$

$$\Rightarrow \text{applied stress} = 4 \times 0.222 \times 40 = \underline{35.5 \text{ kPa}}$$

(15 marks)

(b)
$$\rho_i = \frac{pB(1-v^2)N_p}{E} = \frac{40 \times 8 \times 0.75 \times 0.94}{5 \times 10^6} = 45.1 \text{ mm}$$

(5 marks)**(Total 20 marks)**

Q3.

SLIDING

Combination 1:

From Table 5.1: $\gamma_{G; unfav} = 1.35$; $\gamma_{G; fav} = 1.0$; $\gamma_Q = 1.5$; $\gamma_{\phi} = 1.0$.

Design $\phi = 30^\circ$ (since partial factor of safety for $\phi = 1.0$.)

From chart, $K_a = 0.27$

Pressure at base of wall = $K_a \times \gamma \times h = 0.27 \times 19 \times 5 = 25.7$ kPa

$P_a = 0.5 \times 25.7 \times 5 = 64.2$ kN

Design action, $P_{a; d} = 64.2 \times 1.35 = \underline{86.7}$ kN

Weight of wall, $W = \frac{1}{2}(1.7 + 3.0) \times 5 \times 24 = 282.0$ kN

Design weight, $W_d = 282 \times 1.0 = \underline{282}$ kN

Resistance to sliding, $R_d = W_d \times \tan 30^\circ$ (since $\delta/\phi = 1.0$) = $282 \tan 30 = \underline{162.8}$ kN

Since $R_d > P_{a; d}$ limit state requirement is satisfied.

Over design factor = $162.8 / 86.7 = \underline{1.88}$

(10 marks)

Combination 2:

From Table 5.1: $\gamma_{G; fav} = 1.0$; $\gamma_{G; unfav} = 1.0$; $\gamma_Q = 1.3$; $\gamma_{\phi} = 1.25$

Design $\phi = 24.8^\circ$ (since partial factor of safety for $\phi = 1.25$.)

From chart, $K_a = 0.34$

Pressure at base of wall = $K_a \times \gamma \times h = 0.34 \times 19 \times 5 = 32.3$ kPa

$P_a = 0.5 \times 32.3 \times 5.0 = 80.7$ kN

Design action, $P_{a; d} = 80.7 \times 1.0 = \underline{80.7}$ kN

Weight of wall, $W = \frac{1}{2}(1.7 + 3.0) \times 5 \times 24 = 282.0 \text{ kN}$

Design weight, $W_d = 282 \times 1.0 = \underline{282 \text{ kN}}$

Resistance to sliding, $R_d = W_d \times \tan 24.8^\circ$ (since $\delta/\phi = 1.0$) $= 282 \tan 24.8 = \underline{130.3 \text{ kN}}$

Since $R_d > P_{a,d}$ limit state requirement is satisfied.

Over design factor $= 130.3 / 80.7 = \underline{1.61}$

(5 marks)

OVERTURNING

Combination 1:

From Table 5.1: $\gamma_{G, unfav} = 1.35$; $\gamma_{G, fav} = 1.0$; $\gamma_Q = 1.5$; $\gamma_r = 1.0$.

Design $\phi = 30^\circ$ (since partial factor of safety for $\phi = 1.0$.)

From chart, $K_a = 0.27$

Consider wall as comprising 2 sections:

Area 1: $W_d = \frac{1}{2} \times 1.3 \times 5 \times 24 \times \gamma_{G, fav} = 78.0 \text{ kN}$

Area 2: $W_d = 1.7 \times 5 \times 24 \times \gamma_{G, fav} = 204.0 \text{ kN}$

Active thrust:

$P_a = 0.5 \times 25.7 \times 5 = 64.2 \text{ kN}$

Design action, $P_{a,d} = 64.2 \times 1.35 = \underline{86.7 \text{ kN}}$

The effect of the actions is to cause the overturning moment about the toe of the wall. This is resisted by the stabilising moment from the self-weight of the wall.

Action	Magnitude of Action (kN)	Lever arm (m)	Moment (kNm)
Stabilising:			
Area 1	78.0	$0.67 \times 1.3 = 0.88$	68.6
Area 2	204.0	$1.3 + (1.7/2) = 2.15$	438.6
		Total:	507.2
Destabilising:			
P_a	86.7	$5/3 = 1.67$	144.5
		Total:	144.5

Over design factor = $507.2 / 144.5 = \underline{3.51}$

(10 marks)

Combination 2:

From Table 5.1: $\gamma_{G; fav} = 1.0$; $\gamma_{G; unfav} = 1.0$; $\gamma_Q = 1.3$; $\gamma_r = 1.25$

Design $\phi = 24.8^\circ$ (since partial factor of safety for $\phi = 1.25$.)

From chart, $K_a = 0.34$

Weight and Moments of Wall same as Combination 1 (since $\gamma_{G; fav} = 1.0$ for both)

$$\mathbf{M_{stb;d} = \underline{507.2 \text{ kNm}}}$$

Active thrust:

$$\text{Pressure at base of wall} = K_a \times \gamma \times h = 0.34 \times 19 \times 5 = 32.3 \text{ kPa}$$

$$P_a = 0.5 \times 32.3 \times 5.0 = 80.7 \text{ kN}$$

$$\text{Design action, } P_{a;d} = 80.7 \times 1.0 = \underline{80.7 \text{ kN}}$$

$$\mathbf{M_{dst;d} = 80.7 \times 1.67 = \underline{134.8 \text{ kNm}}}$$

Over design factor = $507.2 / 134.8 = \underline{3.76}$

(5 marks)

(Total 30 marks)